

## STEEL DESIGN

**CODE:** Allowable Stress Design - Ninth Edition

**ANALYSIS TYPE:** Member Verification

**CODE GROUP:**

**MEMBER:** 649

**POINT:** 1

**COORDINATE:** x = 0.00 L = 0.00 m

**LOADS:**

Governing Load Case: 8 PP+SCM+VMAX (1+2+3)\*1.00

**MATERIAL:**

STEEL A572-50  $F_y = 3515.35 \text{ kG/cm}^2$



**SECTION PARAMETERS: TRABE\_1**

d=110.0 cm

b=30.0 cm

tw=1.3 cm

tf=2.2 cm

$A_y = 132.000 \text{ cm}^2$

$I_y = 638684.077 \text{ cm}^4$

$W_{ely} = 11612.438 \text{ cm}^3$

$A_z = 286.000 \text{ cm}^2$

$I_z = 66476.749 \text{ cm}^4$

$W_{elz} = 4431.783 \text{ cm}^3$

$A_x = 406.560 \text{ cm}^2$

$J = 199481.425 \text{ cm}^4$

**MEMBER PARAMETERS:**

$L_y = 0.10 \text{ m}$

$K_y = 1.00$

$KL/ry = 0.25$

$L_b = 0.17 \text{ m}$

UNS = Compact

$L_z = 0.10 \text{ m}$

$K_z = 1.00$

$KL/rz = 0.78$

$C_b = 1.02$

STI = Slender

**INTERNAL FORCES:**

$F_x = 2.58 \text{ T}$

$M_x = 6.55 \text{ T*m}$

$M_y = 216.48 \text{ T*m}$

$M_z = -0.03 \text{ T*m}$

$V_y = -0.05 \text{ T}$

$V_z = -6.27 \text{ T}$

**CALCULATION STRESSES:**

$f_a = 6.34 \text{ kG/cm}^2$

$f_{bcy} = 1864.17 \text{ kG/cm}^2$

$f_{bty} = -1864.17 \text{ kG/cm}^2$

$f_{bcz} = 0.59 \text{ kG/cm}^2$

$f_{btz} = -0.59 \text{ kG/cm}^2$

$f_{vy} = -0.40 \text{ kG/cm}^2$

$f_{vz} = -21.93 \text{ kG/cm}^2$

$f_{vy,mx} = -81.38 \text{ kG/cm}^2$

$f_{vz,mx} = -48.09 \text{ kG/cm}^2$

**ALLOWABLE STRESSES:**

$F_a = 2105.69 \text{ kG/cm}^2$

$F_{bcy} = 2109.21 \text{ kG/cm}^2$

$F_{bty} = 2109.21 \text{ kG/cm}^2$

$F_{bcz} = 2109.21 \text{ kG/cm}^2$

$F_{btz} = 2109.21 \text{ kG/cm}^2$

$F_{vy} = 1406.14 \text{ kG/cm}^2$

$F_{vz} = 1406.14 \text{ kG/cm}^2$

**VERIFICATION FORMULAS:**

$f_a / (0.6 * F_y) + f_{bcy} / F_{bcy} + f_{bcz} / F_{bcz} = 0.89 < 1.05$  ASD (H1-2)

$(f_{vy} + f_{vy,mx}) / F_{vy} = |-0.06| < 1.05$   $(f_{vz} + f_{vz,mx}) / F_{vz} = |-0.05| < 1.05$  ASD (F4)

**Section OK !!!**

**Hand calculations**

According to ASD 9<sup>th</sup> edition Section F4,

$h/tw = 84.61 \leq 380 / \sqrt{F_y}$ ; NO

thus,  $F_v = (F_y / 2.89) C_v \leq 0.4 F_y$

since there are no transverse stiffeners,  $k_v=5.34$

Value of  $C_v=0.72$

This results in  $F_v=871.97 \text{ kg/cm}^2$  instead of  $1406.14 \text{ kg/cm}^2$  used by RSA 2012.