Here are the values Robot calculated after I entered S2, Ss, etc:


Here are my values from an excel spreadsheet:

| Mapped Acceleration Parameters |  |  |  |  | ASCE 7-05 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Location | Latitude | 39.7 |  |  | Section |
| Longitude -121.84 |  |  |  |  |  |
| Site Class | C |  |  |  | 20.3 |
| $\mathrm{S}_{5}$ | 0.598 |  |  |  | 11.4.1 |
| $\mathrm{S}_{1}$ | 0.229 |  |  |  | 11.4.1 |
| $\mathrm{S}_{\mathrm{MS}}$ | 0.791 |  |  |  | 11.4.3 |
| $\mathrm{S}_{\mathrm{M} 1}$ | 0.445 |  |  |  | 11.4.3 |
| $\mathrm{F}_{\mathrm{a}}$ | 1.32274 |  |  |  | 11.4 .3 |
| $\mathrm{F}_{\mathrm{v}}$ | 1.94323 |  |  |  | 11.4 .3 |
| $S_{\text {DS }}$ | 0.527 |  |  |  | 11.4.4 |
| $\mathrm{S}_{\mathrm{D1}}$ | 0.297 |  |  |  | 11.4.4 |
| $T=T_{a}$ | 0.16987 | 5 |  |  | 12.8.2.1 |
| $\mathrm{C}_{\mathrm{t}}$ | 0.02 |  |  | Table | 12.8-2 |
| x | 0.75 |  |  | Table | 12.8-2 |
| $\mathrm{h}_{\mathrm{n}}$ | 17.33 | ft |  |  | 12.8.2.1 |
| $\mathrm{T}_{0}$ | 0.11271 | 5 |  |  | 11.4 .5 |
| $\mathrm{T}_{5}$ | 0.56357 | 5 |  |  | 11.4 .5 |
| $\mathrm{T}_{\mathrm{L}}$ | 16 | $s$ |  |  | 11.4 .5 |
| Occupancy Category | III |  |  | Table | 1-1 |
| I | 1.25 |  |  | Table | 11.5-1 |
| Seismic Category | D |  |  | Table | 11.6-1 |
| $\mathrm{Sa}_{3}$ | 0.527 |  | $\mathrm{T}_{0}<\mathrm{T}<\mathrm{T}_{5}$ |  | 11.4.5 |


| Design Coefficients and Factors |  |  |  | ASCE 7-05 |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Section |
| Response Modification Coefficient, R | 3.5 |  | Table | 12.2-1 |
| System Overstrength Factor, $\Omega$ | 2.5 |  | Table | 12.2-1 |
| Deflection Amplication Factor, $\mathrm{C}_{\mathrm{d}}$ | 2.25 |  | Table | 12.2-1 |
| redundacy factor, $\rho$ | 1.3 |  |  | 12.3.4.2 |
| Seismic Response Coefficient, $\mathrm{C}_{5}$ | 0.188214 |  |  | 12.8.1.1 |
| upper limit for $\mathrm{C}_{5}$ | 0.624409 | $\mathrm{T}<\mathrm{T}_{\mathrm{L}}$ |  | 12.8.1.1 |
|  |  |  |  |  |
| Base Shear, V | 362.6653 | kips |  | 12.8.1 |
| k | 1 |  |  | 12.8 .3 |
| $\mathrm{C}_{\mathrm{vx}}$ | 1 |  |  | 12.8 .3 |
| $\mathrm{F}_{\mathrm{x}}$ | 362.6653 | kips |  | 12.8 .3 |
|  |  |  |  |  |
| Seismic Load Combination |  |  |  |  |
| $1.2+0.2 \mathrm{~S}_{\text {DS }}$ | 1.3054 |  |  | 12.14.3.1 |
| $0.9-0.2 S_{\text {Ds }}$ | 0.7946 |  |  | 12.14.3.1 |
| Strength Design; LRFD |  |  |  |  |
|  |  |  |  |  |
|  |  |  |  |  |
|  |  |  |  |  |
|  |  |  |  |  |

Design Response Spectrum


